

LARGE-SCALE BACKFILLING SIMULATIONS AT ÄSPÖ HRL AND APPLICABILITY OF RESULTS TO CANADIAN REPOSITORY CONCEPTS

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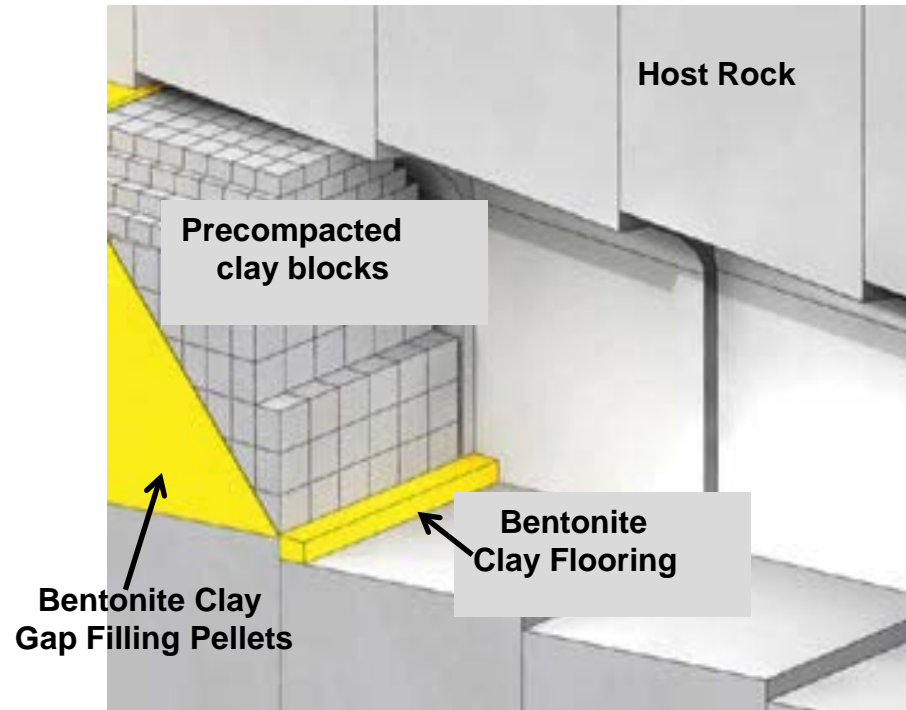
Background

- NWMO's APM approach to permanent disposal of used nuclear fuel includes ongoing evaluation of placement geometries and host media,
- Options considered by NWMO includes granitic host rock and in-floor borehole (IFB) placement of containers holding used nuclear fuel.
- SKB (Sweden) and Posiva (Finland) have selected IFB option as their reference design.
- As part of design process, SKB and Posiva are evaluating effect of water inflow on backfill behaviour.

Testing of Backfilling Concept

- SKB and Posiva have active research programs focused on development of backfill and backfilling technologies.
- One activity is evaluation of backfill in the period immediately following its installation.
- NWMO provided in-kind contributions to the SKB-Posiva BACLO project as part of its ongoing concept evaluation work (Results from small-scale backfilling simulations completed in Canada by AECL).
- Results of several subactivities have been published by SKB, Posiva, NWMO and/or presented at international conferences.

Tunnel Backfilling Using Blocks and Pellets



- Crushed or pelletized swelling clay is used to level floor,
 - Precompacted clay blocks are then installed,
 - Blocks fill the majority of the room volume (>70%), and
 - Remaining gaps are filled with clay pellets.

Purpose of Studies

Evaluate materials and methods for placement tunnel backfilling in a repository in crystalline rock.

Determine the effect of inflowing water on system performance immediately after backfilling.

Regardless of Geological Medium: Backfill needs to be:

- Technically feasible to install (5-8 m/day SKB & Posiva concepts)
- Able to resist disruption by inflowing water during ongoing backfilling operations.
- Able to accommodate inflow of water without having its long-term behaviour compromised (swelling, hydraulic and mechanical)
- Safe, not result in development of conditions that might be dangerous to closure or repository operation.

Aspects Evaluated

Evaluation of behaviour of backfill immediately following its placement has included:

- *Testing placement technologies and determining achievable as-placed backfill densities.*
- *Identifying conditions or processes causing instability of the backfill or that make backfilling operations problematic.*
- *Determining water inflow rate where backfill stability is compromised, specialized water handling, or remediation of adjacent rock is likely to be needed.*
- *Evaluating the effect of water inflow pattern (point sources, dispersed seepage) on backfill behaviour.*
- *Determining effect of isolated and pressurized sections on system behaviour prior to tunnel closure.*

Tests to Evaluate Materials and Interactions



Bench-Scale
(SKB and Posiva)



1/12th Scale
(NWMO)

1/4-Scale
(Posiva and SKB)

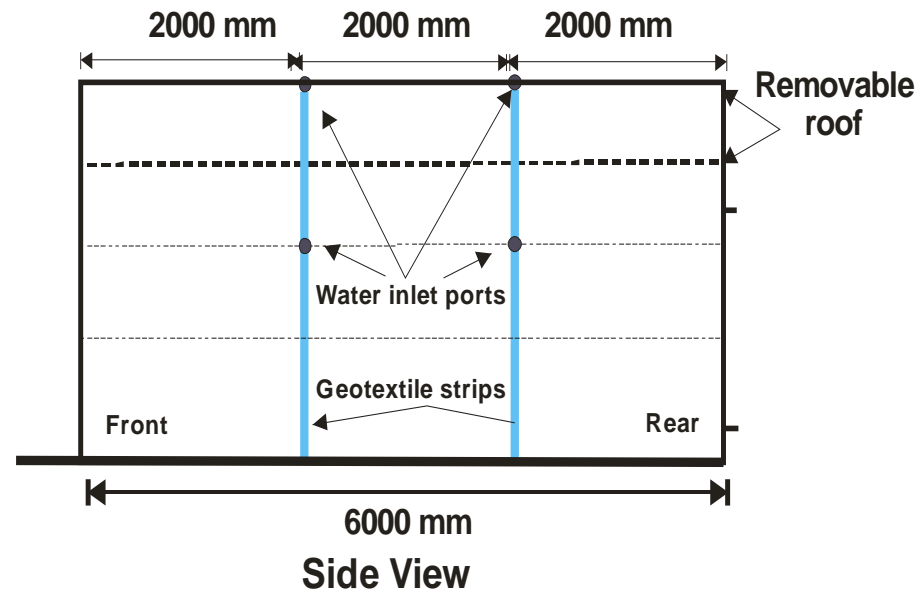
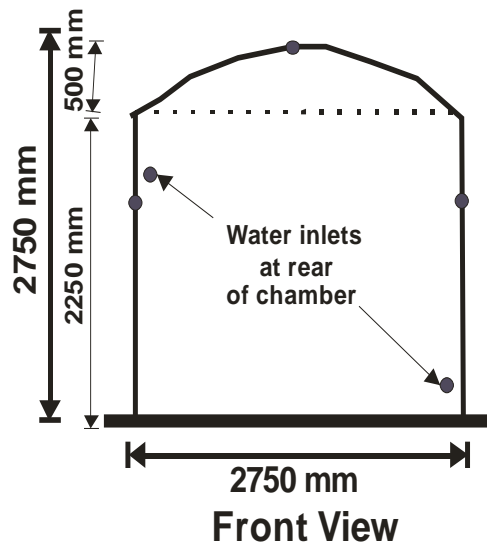


1/2 Scale Mockup
(at Äspö Sweden, Joint SKB/Posiva)

Results of Small-Scale Tests

- Laboratory and Bench-Scale tests undertaken by SKB, Posiva and NWMO indicate that backfill can develop preferential flowpaths
- Pathways are generally at interfaces (rock-pellet; pellet-block)
- At very low inflow ($< \sim 0.1$ l/min) these features were generally non-erosive and were not physically disruptive
- Small flow features can combine into a single pathway, causing increased erosion
- Removal (erosion) of clay can be affected by:
 - Water velocity ($> \text{velocity} > \text{erosion}$),
 - Salinity (higher salinity $> \text{erosion}$),
 - Clay block density, and
 - As-Placed density of backfill pellets.

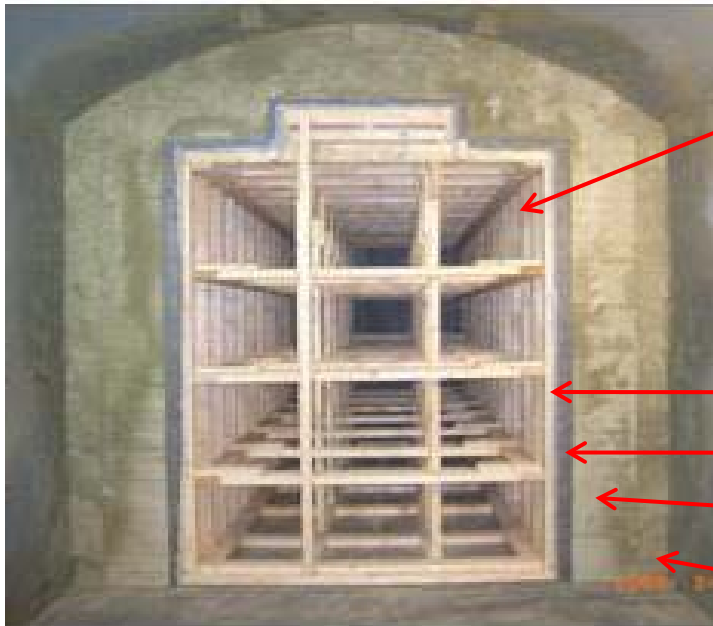
1/2-Scale Chamber Used to Evaluate Post-Placement Backfill Behaviour



Construction of 1/2-Scale Tests

1/2-Scale test design took into account results of previously completed, smaller-scale tests, Specifically that:

- Initial flow is controlled by pellet fill,
- Interior block-fill generally plays little role in water movement.
- Substantial swelling pressures can develop in pellet-filled regions.



Mockups consist of:

- Robust wooden formwork occupying tunnel core,
- Internal formwork protects the test chamber by ability to fail should swelling pressures approach ~ 500 kPa.
- Heavy plastic internal liner
- Bentonite geotextile outer liner,
- 300 mm of precompacted clay blocks,
- 100+ mm of pellet fill at perimeter.

Water Movement and Clay Erosion Caused by Point-Source of Inflow



Initial Appearance



Inflow 0.25 l/min left, 0.5 l/m right
Test duration 168 h,
Initial outflow @ 24 h



Inflow 2.5 l/min right side
Test duration 65 h
Initial outflow @ 0.5 h

Path length ~4 m

Water supplied to rear of chamber

½-Scale Test Results: Part 1: Point Inflow to Tunnel

Incoming water prefers to move along “rock” – clay interface:

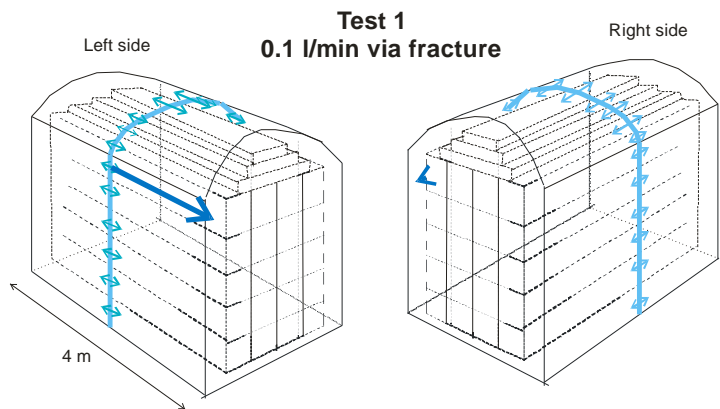
- At $< \sim 0.1$ L/min,
 - Water movement gradually into pellets (and blocks)
 - Exiting water had essentially no suspended or entrained solids.
- At ~ 0.1 to ~ 0.25 L/min,
 - Water movement and erosive action increased with inflow rate.
 - Erosion rate was low, decreasing with time (stabilised flow path).
- For point or combined inflows > 0.5 L/min,
 - Water moves rapidly downstream, only localized water uptake.
 - Considerable erosion of the backfill pellets.
- At $> \sim 2.5$ L/min,
 - Water moves rapidly towards open section of tunnel,
 - Substantial erosion of pellet materials,
 - Erosion of blocks can also occur.

Part 2:

Water Inflow Via Intersecting Fractures

- Effect of a dispersed wetting (e.g. intersection of tunnel by water-bearing fractures)
- Tests simulated the effect of water supply by fracture-only and also effect of subsequent upstream wetting of a hydraulically isolated section of tunnel.
 - Geotextile strips simulated an intersecting fracture,
 - Water was supplied to geotextile-only or in combination with point source(s) at rear of chamber,
 - Resistance of isolated pocket to water entry was monitored as were conditions at the “fracture” features.
 - Ability of the pellet-filled region to “store” water was evaluated.
 - How water moved past gaskets formed by seeping fracture features was monitored.

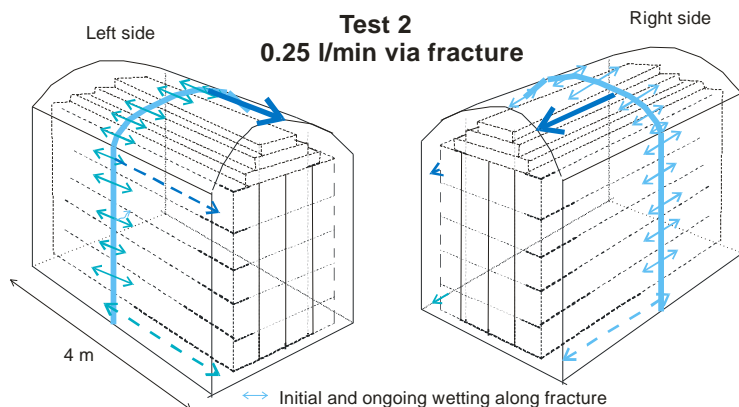
Water Inflow From Intersecting Fracture (inflows 0.1 and 0.25 l/min for ~ 300 h)



↔ Initial and ongoing wetting along fracture
→ Main flow path for exiting water



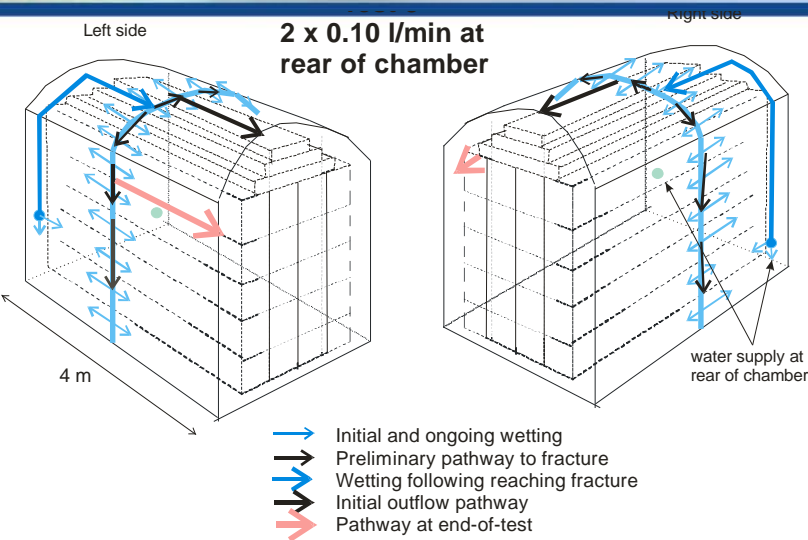
**Relatively uniform water uptake; water movement via a single pathway;
no erosion at 0.1 l/min and discernible erosion at 0.25 l/min**



↔ Initial and ongoing wetting along fracture
→ Very small outflows early in test operation
→ Main flow path for exiting water



Water Movement Past A Seeping Fracture



Downstream of fracture
no block wetting



Block wetting at
fracture location

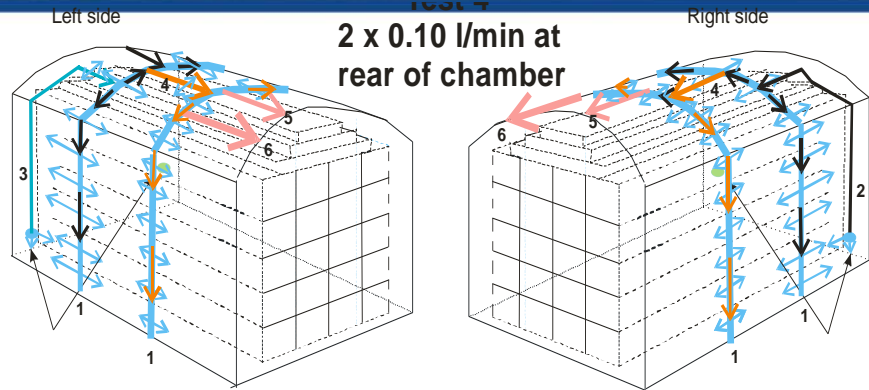
Upstream of fracture,
extensive block wetting



Summary of 270 h Test

- Presence of a fracture-isolated section of tunnel resulted in pressurization of trapped air,
- Outflow was delayed (~5 days) by “gasket” feature,
- Substantial water storage upstream of fracture
- Breaching involved sudden expulsion of air-water-clay mix.
- A highly conductive and erosive flow path was formed.

Water Movement Past Successive Isolating Features



- 1 \leftrightarrow Initial and ongoing wetting via fractures
- 2 \rightarrow Preliminary pathway to fracture via right side
- 3 \rightarrow Preliminary pathway to fracture via left side
- 4 \rightarrow Pathway from inner to outer fracture
- 5 \rightarrow Pathway from outer fracture to front of chamber
- 6 \rightarrow Pathway at end-of-test



Dry blocks at downstream



Limited wetting at inner fracture



Extensive wetting behind inner fracture

Summary of ~325 h Test

- Wetting by fracture isolated a section of tunnel, resulting in pressurized or trapped air,
- Start of outflow was noticeably delayed (~5 days)
- Water storage upstream of inner fracture but not outer,
- Breaching involved sudden expulsion of air-water-clay mix.
- A highly conductive and erosive flow path was formed.



Summary

- Inflowing water has potential to disrupt newly installed backfill,
- Pellet-rock interface seems to be the preferred flow path. For low flow (<0.1 L/min), this does not result in substantial erosion,
- Disturbance is likely at $> \sim 0.25$ L/m inflow via a single pathway,
- Where pellet thickness is low, water may enter block-filled volume, generate flow paths at clay-block or block-block interfaces,
- Seepage from fractures can lead to development of isolated sections of tunnel where pressurized air pockets form,
- Uncontrolled air release prior to installation of the mechanical plug at end of the placement room can cause backfill disruption,
- After plug installation piping, interface flow or trapped air movement should not be a problem.

Relevance to NWMO Repository Concept

- Backfill formulations and installation methods developed and demonstrated in SKB-Posiva Tests have potential for use in NWMO DGR concept.
- Potentially disruptive processes have been identified (high point inflow, development of isolated pockets of compressed air).
- Relevant to NWMO concepts as they have potential to exist in any geological medium where water is entering excavations.
- Reinforces importance of pre-screening deposition tunnels for high-inflow features.
- Highlights importance of inflow water-management during backfilling operations.

 **AECL EACL**

